


TAPE CORRECTIONS

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STANDARD CONDITIONS

- Foot system
 - Temperature - 68° F
 - Tape fully supported
 - Tension – 10 lbs
- Metric system
 - Temperature 20° C
 - Tape fully supported
 - Tension 50 N (Newtons)
 - 1 lb = 4.448 N



COLONIAL AMERICAN
SURVEYOR'S
SEMI-CIRCUMFERENTOR
& CHAIN

ERRORS IN TAPING

- Systematic taping errors
 - Slope
 - Erroneous length
 - Temperature
 - Tension
 - Sag
- Random taping errors
 - Slope
 - Temperature
 - Tension
 - Sag
 - Alignment
 - Marking
 - Plumbing
 - Straightness of tape
 - Observational imperfections



APPLYING CORRECTIONS

$$T = R + C$$

or

$$T = R - E$$

- T = True Value
- R = Field Reading
- C = Correction
- E = Error

TAPING OPERATIONS

- Measuring between points
 - The value R is recorded in the field and the corrections computed
 - T is calculated
- Setting out a value
 - T is now known and the corrections are computed
 - R is calculated to the conditions in the field

SLOPE CORRECTION

- From right triangle geometry
 - H: horizontal distance
 - S: slope distance
 - V: vertical distance
 - θ : vertical angle
 - Z: zenith angle
- $$H = S \cos \theta$$
- $$= S \sin z$$
- $$S^2 = H^2 + V^2 \quad \text{from which}$$
- $$H = \sqrt{S^2 - V^2}$$
- $$V = \sqrt{H^2 - S^2}$$

SLOPE CORRECTION

- Slope expressed as gradient or rate of grade
 - Ratio of vertical distance over horizontal distance
 - Rise over run
 - A +2% slope means 2 units rise in 100 units horizontal
 - A -3.5% slope means 2.5 units fall in 100 units horizontal

SLOPE CORRECTION EXAMPLE

- A road centerline gradient falls from station 0 + 00 (elevation = 564.22') to station 1 + 50 at a rate of -2.5%. What is the centerline elevation at station 1 + 50?
- Difference in elevation:
$$\begin{aligned} \text{Elev}_{\text{Diff}} &= \text{Dist} \left(\frac{\text{rise}}{\text{run}} \right) \\ &= 150' \left(\frac{-2.5'}{100'} \right) = -3.75' \end{aligned}$$
- Elevation at 1 + 50:
$$\begin{aligned} \text{Elev}_{1+50} &= \text{Elev}_{0+00} + \text{Elev}_{\text{Diff}} \\ &= 564.22' - 3.75' = \underline{560.47'} \end{aligned}$$

SLOPE CORRECTION EXAMPLE

- A road centerline runs from 1 + 00 (elevation = 471.37') to station 4 + 37.25 (elevation = 476.77'). What is the slope of the centerline grade line?

- Elevation difference:
$$\begin{aligned} \text{Elev}_{\text{Diff}} &= \text{Elev}_{4+37.25} - \text{Elev}_{1+00} \\ &= 476.377' - 471.37' = +5.40' \end{aligned}$$

- Distance:
$$\begin{aligned} \text{Dist} &= \text{STA}_{4+37.25} - \text{STA}_{1+00} \\ &= 337.25' \end{aligned}$$

- Gradient:
$$\text{Slope} = \left(\frac{\text{Elev}_{\text{Diff}}}{\text{Dist}} \right) \cdot 100 = \left(\frac{+5.40'}{337.25'} \right) \cdot 100 = +1.60\%$$

SLOPE CORRECTION EXAMPLE

- The slope distance between two points is 78.22' and the vertical angle is 1°20'. What is the corresponding horizontal distance?

- Horizontal distance:

$$\cos \theta = \frac{\text{opposite}}{\text{hypotenuse}} = \frac{H}{S}$$

$$\begin{aligned} H &= S \cdot \cos \theta = (78.22') \cos(1^\circ 20') \\ &= \underline{78.20'} \end{aligned}$$

SLOPE CORRECTION EXAMPLE

- The slope distance between two points is 78.22' and the zenith angle is 88°40'. What is the corresponding horizontal distance?

Note: $\sin z = \cos (90^\circ - \theta)$

- Horizontal distance:

$$\sin z = \frac{\text{opposite}}{\text{hypotenuse}} = \frac{H}{S}$$

Same answer as in previous example

$$\begin{aligned} H &= S \cdot \sin z = (78.22') \sin(88^\circ 40') \\ &= \underline{78.20'} \end{aligned}$$

SLOPE CORRECTION EXAMPLE

- A slope rises from one point, a distance of 156.777m, to another point at a rate of +1.5%. What is the corresponding horizontal distance between the points?

- Vertical angle:

$$\tan \theta = \frac{\text{rise}}{\text{run}} = \frac{1.50'}{100'}$$

$$\theta = 0.85937^\circ$$

$$= 0^\circ 51' 34''$$

- Horizontal distance:

$$H = S \cos \theta$$

$$= 156.777 \text{ m} \cdot \cos(0^\circ 51' 34'')$$

$$= \underline{156.759 \text{ m}}$$

SLOPE CORRECTION EXAMPLE

- The slope distance between two points is measured to be 199.908 m and the vertical distance between the points (i.e., the difference in elevation) is +2.435 m. What is the horizontal distance between the points?

- Horizontal distance:

$$H^2 = S^2 - V^2$$

$$H = \sqrt{(199.908 \text{ m})^2 - (2.435 \text{ m})^2}$$

$$= \underline{199.893 \text{ m}}$$

SLOPE CORRECTION

- So far – compute H and V directly
- Can compute correction
- Error due to slope: $E_h = S - H$

- Correction for slope: $C_h = H - S$

- Substitute $H = S \cdot \cos \theta$

- Correction for slope: $C_h = S \cdot \cos \theta - S$
 $= S(\cos \theta - 1)$

SLOPE CORRECTION

- If vertical distance given instead of vertical angle

- Correction:

$$C_h = H - S = \sqrt{S^2 - V^2} - S$$

$$= S \sqrt{1 - \frac{V^2}{S^2}} - S$$

- Use binomial theorem and expand radical $\sqrt{1 - \frac{V^2}{S^2}}$

$$C_h = S \left(1 - \frac{V^2}{2S^2} - \frac{V^4}{8S^4} - \frac{V^6}{16S^6} - \frac{5V^8}{128S^8} - \dots \right) - S$$

- Reducing

$$C_h = -\frac{V^2}{2S} - \frac{V^4}{8S^3} - \frac{V^6}{16S^5} - \frac{5V^8}{128S^7} - \dots$$

SLOPE CORRECTION

- Generally, only first term used

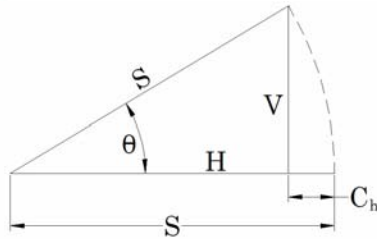
- Valid for slope < 10-15%

- Where required precision < 1:15,000

$$C_h = -\frac{V^2}{2S}$$

- If more precision necessary, additional terms required

SLOPE CORRECTION (ALTERNATIVE)



- Height from Pythagorean Theorem

$$V^2 = S^2 - H^2$$

$$= (S + H)(S - H)$$
- If slope not too large, slope and horizontal distances nearly the same

$$S + H = 2S \quad (\text{approx.})$$

SLOPE CORRECTION (ALTERNATIVE)

- Then,

$$V^2 = 2S(S - H) \Rightarrow \frac{V^2}{2S} = S - H \quad (\text{approx})$$

- Correction for slope: $C_h = H - S$

- Then,

$$C_h = -\frac{V^2}{2S}$$

SLOPE CORRECTION UNCERTAINTY

- Uncertainty in horizontal distance found by error propagation

$$e_h^\ell = \pm \ell (\sin \theta) e_\theta \quad \text{or} \quad e_h^\ell = \pm \frac{V}{\ell} e_V$$

- e_h^ℓ = error in one tape length due to uncertainty in θ
- ℓ = tape length (slope)
- e_θ = error in vertical angle
- e_V = uncertainty in elevation difference

SLOPE CORRECTION UNCERTAINTY

- If slope uniform over entire length of line, and θ or V is over entire length, then $L = \ell \cdot n$ and

$$e_h^L = \pm L (\sin \theta) e_\theta \quad \text{or} \quad e_h^L = \pm \frac{V}{L} e_V$$

- Otherwise use rule for addition of random error:

$$e_h^L = \pm \left[(g_1 e_{V_1})^2 + (g_2 e_{V_2})^2 + \dots + (g_n e_{V_n})^2 \right]^{1/2}$$

- g_1, g_2, \dots, g_n = slope (V/ℓ) for particular length $\ell_1, \ell_2, \dots, \ell_n$

SLOPE CORRECTION EXAMPLE

- A measurement was made along a line inclined by $3^\circ 22'$. The slope distance is 3,236.86'. What is the horizontal distance?
- Correction for slope

$$C_h = S(\cos \theta - 1)$$

$$= 3,236.86'[(\cos 3^\circ 22') - 1]$$

$$= -5.586'$$
- Horizontal distance

$$T = R + C$$

$$= 3,236.86' + (-5.59')$$

$$= \underline{3,231.27'}$$

SLOPE CORRECTION EXAMPLE

- If the uncertainty in θ was $1'$, what is the uncertainty in the entire length?
- Uncertainty:

$$e_h^L = \pm L(\sin \theta)e_\theta$$

$$= \pm(3,236.86')\sin 3^\circ 22' \left(1' \frac{2\pi^{rad}}{180^\circ}\right)$$

$$= \pm 0.06'$$
- Corrected distance:

$$T = R + C = \underline{3,231.27' \pm 0.06'}$$

STANDARD LENGTH CORRECTION

- Tapes assumed to be correct as manufactured
 - Exception is for precise taping
 - Wear – tapes become kinked and stretched
- Correction:

$$C_\ell = \ell_s - 100' \quad \text{or} \quad C_\ell = \ell_s - 30m$$

- ℓ_s = calibrated value of tape
- Discrepancy found through tape comparison to a standard tape

STANDARD LENGTH CORRECTION

- Estimate of error in calibration: e_ℓ
- Uncertainty in length of line due to error in calibration:

$$e_\ell^L = \pm e_\ell \cdot n$$

- n = number of tape lengths
- Error tends not to compensate



STANDARD LENGTH EXAMPLE

- A measurement was recorded as 171.278 m with a 30-m tape that was only 29.996 m under standard conditions. What is the corrected measurement?
- **Correction per tape length:** $C_\ell = \ell_s - 30\text{ m} = 29.996\text{ m} - 30\text{ m}$
 $= -0.004\text{ m}$
- **Number of tape lengths:** No. Tape Lengths = $\frac{\text{Dist}}{30\text{ m}} = \frac{171.278\text{ m}}{30\text{ m}}$
 $= 5.71$
- **Total correction:** $C^L = C_\ell \cdot \text{No. Tape Lengths}$
 $= (-0.004\text{ m}) \cdot (5.71\text{ tape lengths}) = -0.023\text{ m}$
- **Corrected distance:** $T = R + C^L = 171.278\text{ m} + (-0.023\text{ m})$
 $= \underline{171.255\text{ m}}$

STANDARD LENGTH EXAMPLE

- If the uncertainty in the calibrated length of the line is $\pm 0.0012\text{ m}$, what is the uncertainty in the entire length of the line?
- **Total uncertainty is:**
 $e_\ell^L = \pm e_\ell \cdot n = \pm(0.0012\text{ m}) \cdot (5.71\text{ tape lengths})$
 $= \pm 0.007\text{ m}$
- **The length of the line is then**
 $\text{Dist} = \underline{171.255\text{ m} \pm 0.007\text{ m}}$

STANDARD LENGTH EXAMPLE

- A surveyor is using a tape whose standard length is 100.02'. Two pins need to be set out 600.00' apart. What is the field measurement needed to set out the correct length?

- Correction per tape length $C_\ell = \ell_s - 100' = 100.02' - 100'$
 $= 0.02'$ per tape length

- Total correction $C_\ell^L = C_\ell \cdot n = (0.02')(6 \text{ tape lengths})$
 $= 0.12'$

- Field reading $R = T - C = 600.00' - 0.12'$
 $= \underline{599.88'}$

TEMPERATURE

- Tape susceptible to dimensional change due to variation in temperature
- Correction:

$$C_T = \ell \alpha (T - T_S)$$

– C_T = correction due to temperature

ℓ = length of tape

α = coefficient of thermal expansion

$\alpha = 0.00000645$ per 1°F

$\alpha = 0.0000116$ per 1°C

T = field temperature

T_S = standard temperature (Normally 68°F or 20°C)



TEMPERATURE

- Uncertainty – differentiate w.r.t. T $\frac{\partial C_T}{\partial T} = \ell \alpha$

- From error propagation, error in one tape length due to error in temp is:

$$e_T^\ell = \pm \sqrt{(\ell \alpha)^2 e_T^2} = \pm \ell \alpha e_T$$

- e_T^ℓ = error in one tape length
- e_T = error in determining temp.

- Total error of length of line:

$$e_T^L = \pm \ell \alpha e_T \sqrt{n}$$

TEMPERATURE UNCERTAINTY

- Last formula – assumes uncertainty same for each length
 - Generally average temp. used over line
 - Thus, error would not tend to compensate

- Total error estimate: $e_T^L = \pm \ell \alpha e_T n$

- Recognizing $L = \ell n$,

$$e_T^L = \pm L \alpha e_T$$

TEMPERATURE CORRECTION EXAMPLE

- Line is measured as 876.42 m. The field temperature is 24° C. A 30-m tape with correct length at 20° C was used. Find the corrected length of the line.

- The correction per tape length:

$$C_T = \ell \alpha (T - T_s) = (30 \text{ m})(0.0000116 \text{ per } 1^\circ\text{C})(24^\circ - 20^\circ) \\ = 0.001 \text{ m/tape length}$$

- The correction for the line:

$$n = \frac{L}{30 \text{ m}} = \frac{876.42 \text{ m}}{30 \text{ m}} = 29.214 \text{ tape lengths}$$

$$C_T^L = n C_T = (29.214 \text{ tape lengths}) \cdot (0.001 \text{ m/tape length}) = 0.04 \text{ m}$$

- The corrected length of line:

$$T = R + C_T^L = 876.42 \text{ m} + 0.04 \text{ m} \\ = \underline{876.46 \text{ m}}$$

TEMPERATURE CORRECTION EXAMPLE

- If $e_T = 1^\circ \text{ C}$, the total error estimate is

$$e_T^L = \pm L \alpha e_T = (876.42 \text{ m}) \cdot (0.0000116) \cdot (0.0000116) \cdot 1^\circ\text{C} \\ = \underline{\pm 0.01}$$

- The distance is: $876.46 \text{ m} \pm 0.01 \text{ m}$

TEMPERATURE CORRECTION EXAMPLE

- You must lay out two points in the field that will be exactly 100.000 m apart. Field conditions indicate that the temperature of the tape is 27° C. What distance will be laid out?

- Correction for temperature:

$$C_T = \ell \alpha (T - T_s) = (100.000 \text{ m})(0.0000116 \text{ per } 1^\circ\text{C})(27^\circ - 20^\circ) \\ = +0.008 \text{ m}$$

- Distance used to lay out:

$$R = T - C = 100.000 \text{ m} - 0.008 \text{ m} \\ = \underline{99.992 \text{ m}}$$

TENSION CORRECTION

- Applied stress of wire – force per unit area
- Resultant strain – elongation per unit length

$$\text{Stress} = \frac{P}{A} \quad \text{Strain} = \frac{eL}{\ell}$$

- where:
 - P = tension (force)
 - A = cross-sectional area of tape
 - eL = elongation produced by tension
 - ℓ = length of tape

TENSION CORRECTION

- Hooke's Law – P
proportional to eL
and stress
proportion to strain

$$\frac{P}{A} \propto \frac{eL}{\ell}$$

- Insert constant: $\frac{P}{A} = E \frac{eL}{\ell}$ or $E = \frac{P\ell}{A(eL)}$

Proportionality constant (E) called Young's modulus of elasticity

TENSION CORRECTION

- Error found by comparing elongation produced from standardized tension (P_s) and field tension (P)

$$eL = \frac{P\ell}{AE} \quad \text{and} \quad eL = \frac{P_s\ell}{AE}$$

- Correction: $C_p^{\ell} = \frac{P\ell}{AE} - \frac{P_s\ell}{AE} = \frac{(P - P_s)\ell}{AE}$

$$\text{or } C_p^L = \frac{(P - P_s)L}{AE}$$

- Young's modulus of elasticity normally between 28,000,000 psi to 30,000,000 psi

TENSION CORRECTION

- Uncertainty – only variable: field tension

$$e_p^l = \pm \left(\frac{l}{AE} \right) e_p$$

where: e_p^l = error in one tape length
 e_p = error in determining tension

- Error for total length of line (error propagation)

$$e_p^L = \pm \left(\frac{l}{AE} \right) e_p \sqrt{n}$$

TENSION CORRECTION EXAMPLE

- Given: $A = 0.0040$ sq. in $P = 25$ lbs
 $E = 29,000,000$ psi $P_s = 15$ lbs
 $L = 1,000.00'$

- Correction for tension:

$$C_p^L = \frac{(P - P_s)L}{AE} = \frac{(25 - 15 \text{ lbs})(1,000.00')}{(0.0040 \text{ sq in})(29,000,000 \text{ psi})}$$

$$= 0.086'$$

- Corrected distance:

$$T = R + C = 1,000.00' + 0.09'$$

$$= \underline{1,000.09'}$$

TENSION CORRECTION EXAMPLE

- If the uncertainty in P is 1-lb, what is the uncertainty due to tension?

- Uncertainty per tape length:

$$e_p^{\ell} = \pm \left(\frac{\ell}{AE} \right) e_p = \pm \left[\frac{100.00'}{(0.0040 \text{ sq in})(29,000,000 \text{ psi})} \right]$$

$$= \pm 0.0009'$$

- Total uncertainty:

$$e_p^L = \pm e_p^{\ell} \sqrt{n} = \pm (0.0009') \sqrt{10}$$

$$= \pm 0.003'$$

- Distance = 1,000.09' \pm 0.003'

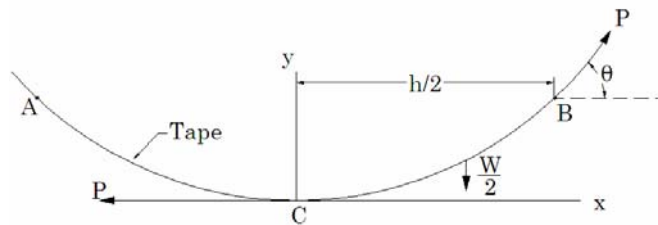
TENSION CORRECTION

- Cross-sectional area
 - Measured with micrometer
 - Taken from manufacturer's specifications
 - Computed from:

$$\text{Tape area} = \frac{\text{weight}}{(\text{length})(\text{specific weight of tape steel})}$$

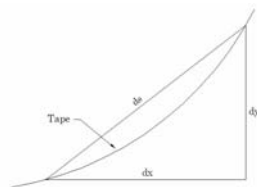
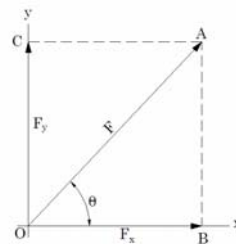
SAG CORRECTION

- Tape supported at ends will sag in center
- Amount of sag depends on
 - Weight of tape per unit length
 - Applied tension
- Arc forms catenary curve or approx. parabola



SAG CORRECTION

- F is force and the components are shown here – can distinguish between forces in x and y directions
- Differentiation of equation of parabola gives slope at support B
- Tape forms differentially short segments of curve – “ds” found by differentiation
- Integrate to find total length of curve
- Horizontal force will approach tension and horizontal distance approaches the curve



SAG CORRECTION

- Correction for sag given as:

$$C_s = -\frac{W^2 \ell}{24P^2} \quad \text{or} \quad C_s^L = -n \left(\frac{W^2 \ell}{24P^2} \right)$$

- Also expressed in terms of weight per foot

$$C_s = -\frac{w^2 \ell^3}{24P^2} \quad \text{or} \quad C_s^L = -n \left(\frac{w^2 \ell^3}{24P^2} \right)$$

- Sag correction always negative
- Sag varies with tension

SAG CORRECTION

- Uncertainty – take derivative of correction equation w.r.t. P and use general propagation formula

$$e_s^\ell = \pm \frac{W^2 \ell}{12P^3} e_P \quad \text{or} \quad e_s^\ell = \pm \frac{w^2 \ell^3}{12P^3} e_P$$

- Assuming same conditions for all tape lengths, error due to sag for total length:

$$e_s^L = \pm \frac{W^2 \ell}{12P^3} e_P \sqrt{n} \quad \text{or} \quad e_s^L = \pm \frac{w^2 \ell^3}{12P^3} e_P \sqrt{n}$$

SAG CORRECTION EXAMPLE

- A 100' steel tape weights 0.02 lbs/ft and supported at the ends only with a tension of 12 lbs. A distance of 350.00' was measured. What is the correction for sag?

- Correction per tape length (100') is:

$$C_s = -\frac{w^2 \ell^3}{24P^2} = -\frac{(0.02 \text{ lbs/ft})^2 (100.00')^3}{24(12 \text{ lbs})^2}$$

$$= -0.116'$$

- Correction for 50' section is

$$C_s = -\frac{w^2 \ell^3}{24P^2} = -\frac{(0.02 \text{ lbs/ft})^2 (50.00')^3}{24(12 \text{ lbs})^2}$$

$$= -0.014'$$

- Total correction:

$$C_s^L = (-0.116') \cdot 3 + (-0.014') = -0.36'$$

SAG CORRECTION EXAMPLE

- If the uncertainty in tension was 1 lb., what is the uncertainty in the total length?

- For 3 full tape lengths

$$e_s^{300'} = \pm \frac{w^2 \ell^3}{12P^2} e_p \sqrt{n} = \pm \left[\frac{(0.02 \text{ lbs/ft})^2 (100.00')^3}{12(12 \text{ lbs})^3} \right] (1 \text{ lb}) \sqrt{3} = \pm 0.033'$$

- For 50' tape length

$$e_s^{50'} = \pm \frac{w^2 \ell^3}{12P^2} e_p = \pm \left[\frac{(0.02 \text{ lbs/ft})^2 (50.00')^3}{12(12 \text{ lbs})^3} \right] (1 \text{ lb}) = \pm 0.002'$$

- For full length:

$$e_s^L = e_s^{300'} + e_s^{50'} = \pm 0.04'$$

- Distance:

$$T = R + C = 350.00' + (-0.36')$$

$$= \underline{349.64' \pm 0.04'}$$

NORMAL TENSION

- Use tape correction to negate effects of sag in tape
- Make error in sag = correction for tension

$$\frac{w^2 \ell^3}{24P^2} - \frac{(P - P_s)\ell}{AE} = 0$$

- Define normal tension $P \rightarrow P_n$

$$\frac{w^2 \ell^3}{24P_n^2} - \frac{(P_n - P_s)\ell}{AE} = 0$$

⋮

$$P_n = \frac{0.204 w \ell \sqrt{AE}}{\sqrt{P_n - P_s}}$$

NORMAL TENSION

- Note that normal tension (P_n) on both sides of equation
- Use as a first approximation

$$P_n = \sqrt[3]{\frac{AEW^2}{15}}$$

- Then take this value for P_n into the previous equation and solve for new normal tension
- Continue until difference below criteria

NORMAL TENSION EXAMPLE

- Find the normal tension given:

$$A = 0.0040 \text{ sq. in.} \quad W = 1.3 \text{ lbs}$$

$$E = 29,000,000 \text{ psi} \quad P_s = 15 \text{ lbs}$$

- Initial estimate of normal tension

$$P_n = \sqrt[3]{\frac{AEW^2}{15}} = \sqrt[3]{\frac{(0.0040 \text{ sq in})(29,000,000 \text{ psi})(1.3 \text{ lbs})^2}{15}} = 24 \text{ lbs}$$

- Adjusted value for normal tension

$$P_n = \frac{0.204 W \sqrt{AE}}{\sqrt{P_n - P_s}} = \frac{(0.204)(1.3 \text{ lbs})\sqrt{(0.004 \text{ sq in})(29,000,000 \text{ psi})}}{\sqrt{24 \text{ lbs} - 15 \text{ lbs}}} = 30 \text{ lbs}$$

- Use mean value of the last 2 P_n values: 27 lbs

$$P_n = \frac{0.204 W \sqrt{AE}}{\sqrt{P_n - P_s}} = \frac{(0.204)(1.3 \text{ lbs})\sqrt{(0.004 \text{ sq in})(29,000,000 \text{ psi})}}{\sqrt{27 \text{ lbs} - 15 \text{ lbs}}} = 26 \text{ lbs}$$

INCORRECT ALIGNMENT

- May occur when more than one tape length measured in field
- Error: random in nature
systematic in effect
- Lateral displacement from true line causes systematic error

$$E_a^\ell = \frac{d_a}{2\ell}$$

- Correction for alignment for entire length found by

$$C_a^L = -n E_a^\ell$$

E_a^ℓ = alignment error

d_a = lateral displacement

TAPE NOT STRAIGHT

- Taping in brush and when wind blowing
 - Impossible to have all parts in perfect alignment
- Error systematic & variable
 - Same as measuring with tape that is too short
- Amount of error
 - Less if bend in in center
 - Increases as it gets closer to ends
- Reduced by careful field procedures

IMPERFECTIONS IN OBSERVATIONS

- Personal errors or blunders
 - Plumbing
 - Marking tape ends with tape fully supported
 - Adding or dropping full tape length
 - Adding a foot or decimeter
 - Other points incorrectly taken as end mark on tape
 - Reading numbers incorrectly
 - Calling numbers incorrectly or not clearly

| SPECIFICATION FOR 1/5,000 TAPING | | |
|---|-----------------------------|----------|
| Source of Error | Max Effect on 1 Tape Length | |
| | 100 ft | 30 m |
| Temp estimated to closest 7° F | ±0.005' | ±0.0014m |
| Tension known within 5 lbs | ±0.006' | ±0.0018m |
| Slope errors no larger than 1'/100' | ±0.005' | ±0.0015m |
| Alignment errors no larger than 0.5'/100' | ±0.001' | ±0.0004m |
| Plumbing & marking errors max 0.015'/100' | ±0.015' | ±0.0046m |
| Length of tape known within ±0.005' | ±0.005' | ±0.0015m |

SPECIFICATION FOR 1/5,000 TAPING

- Total random error in 1 tape length = $\sqrt{\sum \text{errors}^2}$

$$\sum E = \sqrt{0.000337} = 0.018' \quad \sum E = \sqrt{0.0000031} = 0.0056\text{m}$$

$$\text{accuracy} = \frac{0.018'}{100'} = \frac{1}{5,400} \quad \text{or} \quad \text{accuracy} = \frac{0.0056\text{m}}{30\text{m}} = \frac{1}{5,400}$$

- This assumes systematic errors already corrected

| Source | Error of 0.01'/100' caused by | Makes tape too: | Importance in 1:5,000 taping | Procedure to reduce or eliminate | |
|--------------------------------|----------------------------------|-----------------|------------------------------|--|--|
| Tape not of standard length | - | Long or short | Usually small – check | Standardize and apply correction | |
| Temperature | 15° F | Long or short | Only in hot or cold weather | Measure temp & apply correction Use invar tape | |
| Change in tension | 15 lbs | Long or short | Negligible | Apply correction; Use spring balance | Use normal tension Use standard tension |
| Sag | $\Delta P = 0.6$ lb too small | Short | Large esp. for heave tape | Apply correction; Use only fully supported | |
| Slope | 1.4' in h 0°48' in θ | Short | | At breaks in slope, determine differences in height; apply correction | |
| Imperfect horizontal alignment | 1.4' | short | Not serious | Use reasonable care in alignment; keep tape taut and reasonably straight | |

| Source | Governing condition causes and manner of cumulation | Estimated value per tape length |
|---|--|---------------------------------|
| Plumbing to mark tape ends | Rugged terrain, breaking tape often; cumulates randomly $\propto \sqrt{n}$ | 0.05' – 0.10' |
| Marking tape ends with tape fully supported | Tape graduated to hundredths of ft; cumulates randomly $\propto \sqrt{n}$ | 0.01' |
| Applying tension | Change in sag correction due to variation in tension of ± 2 lb from standard tension; cumulates $\propto \sqrt{n}$ | 0.01' |
| Determining elevation diff or slope angle | In h = $\pm 0.8'$ in $\theta = 0^\circ 28'$; Cumulates $\propto n$ | 0.050' |
| standardization | Field tapes compared to standardized tape kept in office; cumulates $\propto n$ | 0.005' |